## Engineers, Planners \& Landscape Architects

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## Mill Run Extension-Phases 7 and 8

Revised Transportation Impact Statement

November 6, 2023
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Reference: Revised Transportation Impact Statement Mill Run Extension - Phases 7 and 8
Town of Mississippi Mills, County of Lanark
Our File No.: 121125
This Revised Traffic Impact Statement has been prepared in support of the Draft Plan of Subdivision application for the Mill Run Extension, Phases 7 and 8, located north of Leishman Drive west of Ramsay Concession 11A and Martin Street.

This study provides a description of the development proposal, a summary of the existing conditions, and an updated estimate of the projected site traffic during the weekday AM and PM peaks. The internal road pattern and access configuration are also assessed.

If you have any questions or comments regarding this report, please feel free to contact Brad Byvelds, or the undersigned.

Yours truly,
NOVATECH



Trevor Van Wiechen, M.Eng. E.I.T.| Transportation

### 1.0 PROPOSED DEVELOPMENT

The subject site has an area of approximately 5.6 hectares. The proposed development consists of 47 single detached housing units, 18 semi-detached housing units, and 60 townhouses. The development includes an extension of Sadler Drive to the north, two streets to the east of Sadler Drive, and a street to the west of Sadler Drive that ends in a cul-de-sac. A copy of the proposed Draft Plan and Concept Plan is shown in Appendix A.

### 2.0 EXISTING CONDITIONS

### 2.1 Roadways and Intersections

Sadler Drive is a local two-lane urban road with sidewalks on either side and runs in the north-south direction. The speed limit is not posted and therefore has a regulatory speed limit of $50 \mathrm{~km} / \mathrm{h}$ under the Ontario Highway Traffic Act.

The Ottawa Street at Sadler Drive/Industrial Drive intersection is a four legged signalized intersection. The westbound and eastbound legs of Ottawa Street have a turn lane, a through lane, and a shared through/right turn lane. The southbound leg of Sadler Drive has a left turn lane and a shared through/right turn lane. The northbound leg of Industrial Drive has a shared through/left turn lane and a channelized right turn lane. All legs of the intersection have standard crosswalks.
Figure 1: Key Plan


### 2.2 Traffic Volumes

In February 2020, Parsons prepared a Traffic Safety Review memorandum for the Municipality of Mississippi Mills. As part of this memorandum, weekday ten hour turning movement counts were conducted in November 2019 at the nearby intersections of Sadler Drive/Industrial Drive/Ottawa Street, Menzie Street/Paterson Street/Ottawa Street and Main Street/Martin Street/Ottawa Street. Volumes from the 2019 traffic count were then factored by an annual $2 \%$ growth rate to adjust for 2023 traffic volumes. A copy of the Parsons November 2019 traffic count data is included in Appendix B and the factored 2023 traffic volumes for the Sadler Drive/Industrial Drive/Ottawa Street are shown in Figure 2.

Figure 2: Factored Traffic Volumes


Trips generated by the proposed development have been estimated using relevant peak hour trip generation rates identified in the Institute of Transportation Engineers (ITE) Trip Generation Manual, $11^{\text {th }}$ Edition. The estimated peak hour vehicle trips generated by the proposed development during the weekday AM and PM peak hours are outlined in the following table.

Table 1: Trip Generation

| Land Use | ITE | Dwelling | AM Peak (vph ${ }^{1}$ ) |  |  | PM Peak (vph) |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Code | Units | IN | OUT | TOTAL | IN | OUT | TOTAL |  |
| Single-Family Detached | 210 | 47 | 9 | 28 | 37 | 31 | 18 | 49 |  |
| Semi-Detached | 215 | 18 | 1 | 3 | 4 | 4 | 3 | 7 |  |
| Townhouse | 220 | 60 | 10 | 31 | 41 | 29 | 17 | 46 |  |
| TOTAL |  |  | $\mathbf{2 0}$ | $\mathbf{6 2}$ | $\mathbf{8 2}$ | $\mathbf{6 4}$ | $\mathbf{3 8}$ | $\mathbf{1 0 2}$ |  |

1. $\mathrm{vph}=$ vehicles per hour

The assumed distribution of trips generated by the proposed development has been estimated based on the local and commuter traffic patterns. The trip distribution assumptions for trips generated by the proposed development are as follows:

- $40 \%$ to/from the east
- $20 \%$ to/from the south
- $40 \%$ to/from the west

Using the trips generated and the assumed trip distribution the following Figure 3 was created to show the generated turning movements at the Sadler Drive/Ottawa Street/Industrial Drive intersection. Total traffic volumes at this intersection are shown in Figure 4.
Figure 3: Site Generated Traffic


Figure 4: Total Traffic


### 4.0 IMPACT ANALYSIS

Based on the intersection analysis presented in the Parsons Traffic Safety Review Memorandum, the Ottawa Street/Industrial Street/Sadler Street intersection operates with a LOS A during the AM peak hour and a LOS B during the PM peak hour. Relevant excerpts from the Parsons Memorandum are provided in Appendix C.

During the weekday AM and PM peak hours, the additional trips generated by the proposed development equate to an overall increase of $6-7 \%$ to the existing traffic volumes at the Sadler Drive/Ottawa Street/Industrial Drive intersection. The additional traffic is considered relatively insignificant compared to the typical capacity of the signalized intersection.

The increase in traffic volumes is not anticipated to have a significant impact to the intersection operations presented in the Parsons Memorandum.

### 5.0 ON-SITE DESIGN

The proposed access to the development is a northern extension of Sadler Drive and creates a 3way intersection where Sadler Drive meets Leishman Drive. The proposed extension of Sadler Drive is roughly 200 m long and has a 20 m right-of-way (ROW) which is consistent with the existing portion of Sadler Drive south of the proposed development.

The proposed internal road network includes Streets 1, 2, and 3. Streets 1 and 2 run east-west between Sadler Drive and the eastern edge of Phase 7. Street 3 begins at the intersection of Sadler Drive and Street 2 and heads westbound and ends in a cul-de-sac. All three internal streets have a 18 m wide ROW.

The TAC Geometric Design Guide specifies that for local roads three-legged intersections must be spaced 40 m apart while four-legged intersections need to be spaced 60 m apart. As the internal roadways are spaced roughly 80 m apart from each other and Street 1 is roughly 80 m from Leishman Drive this satisfies the required distance for all intersections.

Sidewalks have been proposed along both sides of Sadler Drive, the south side of Street 1, and the north side of Street 2 and 3. Sidewalks were placed to provide connectivity to the residential development to the south as well as the Mill Run Park. A new pathway will be provided within Servicing Block 63 shown on the Draft Plan to connect to the pathway as part of Phase 4 of the Mill Run subdivision. The pathway along the north and west edge of the existing stormwater management pond will be removed and replaced with a new pathway connecting the terminus of Street 3 to Sadler Drive.

Side street stop control on the minor street is proposed at each of the proposed intersections.
Within the permanent servicing block to the east of Phase 7 there is a proposed temporary road connection between Streets 1 and 2. The temporary road connection will exist until future roadways that are part of any future development to the east are constructed. The temporary road connection will allow emergency vehicles to turn around and exit the subdivision. A sidewalk on the east side of the temporary road connection will be constructed and will remain once the temporary road connection is no longer needed.

A cross section for the temporary road connection is provided in Figure 5.
The proposed network and pathways plan is included in Figure 6.



### 6.0 CONCLUSIONS AND RECOMMENDATIONS

Based on the foregoing, the conclusions and recommendations of this Transportation Impact Statement can be summarized as follows:

- The marginal increase in traffic volumes attributable to the additional trips generated by the proposed development is not anticipated to have a significant impact to the intersection operations at the Ottawa Street/Industrial Street/Sadler Drive intersection.
- All internal roadways meet TAC requirements as intersections have appropriate spacing.
- The internal sidewalk network will provide connectivity to the residential development to the south as well as the Mill Run Park. The pathway along the northern edge of the existing stormwater management pond will be removed and replaced with a new pathway connecting the terminus of Street 3 to Sadler Drive.
- Within the permanent servicing block to the east of Phase 7 there is a proposed temporary road connection between Streets 1 and 2. The temporary road connection will exist until future roadways that are part of any future development to the east are constructed. The temporary road connection will allow emergency vehicles to turn around and exit the subdivision. A sidewalk on the east side of the temporary road connection will be constructed and will remain once the temporary road connection is no longer needed.

The proposed development is recommended from a transportation perspective.
Yours truly,

## NOVATECH

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## APPENDIX A

## Draft Plan \& Concept Plan




## APPENDIX B

## Traffic Count Data

Turning Movement Count Summary, AM and PM Peak Hour Flow Diagrams

## Industrial Drive/Sadler Drive \& Ottawa Street

## Almonte, ON



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## APPENDIX C

## Excerpts From the Parsons Traffic Safety Review Memorandum

### 3.3. INTERSECTION OPERATION PERFORMANCE

In the following section, the operational capacity of study area intersections will be assessed using Synchro v10 analysis software. The purpose of this analysis is to identify whether there is vehicular congestion that may contribute to safety concerns. The peak hour traffic volumes from Figure 5 were entered and modelled in Synchro. The criteria for the analysis have been summarized below.

### 3.3.1. INTERSECTION ANALYSIS CRITERIA

For signalized intersections, the Level of Service (LOS) defines operational conditions within a traffic stream and their perception by motorists. LOS 'A' represents the best operating conditions and LOS 'E' represents the level which the intersection or an approach to the intersection is carrying the maximum traffic volume that can theoretically be accommodated. LOS ' $F$ ' indicates that the intersection is operating beyond its theoretical capacity.

For the purposes of this analysis, the City of Ottawa criteria for LOS has been referenced. These criteria were developed as part of the Transportation Impact Assessment Guidelines, which relate a LOS designation to be defined range. These criteria are as follows:

Table 2: LOS Criteria for Signalized Intersections

| LOS | Volume to Capacity Ratio <br> $(\mathrm{V} / \mathrm{c})$ |
| :---: | :---: |
| A | 0 to 0.60 |
| B | 0.61 to 0.70 |
| C | 0.71 to 0.80 |
| D | 0.81 to 0.90 |
| E | 0.91 to 1.00 |
| F | $>1.00$ |

A LOS 'D' or better is considered acceptable operations based on City of Ottawa Standards. Based on these criteria, the operational capacity at the study area intersections were assessed in the following section.

### 3.3.2. INTERSECTION ANALYSIS RESULTS

Table 3 provides a summary of the existing traffic operations at the study area intersections. The signalized intersections were assessed in terms of the volume-to-capacity ( $\mathrm{v} / \mathrm{c}$ ) ratio and the corresponding Level of Service (LOS) for the critical movement(s) and for the entire intersection, the latter was assessed based on weighted v/c ratio. The Synchro model output of existing conditions is provided within Appendix E .

| Intersection | Weekday AM(PM) Peak Hour Operational Results |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Critical Movement |  |  | Intersection |  |  |
|  | max. v/c | LOS | Movement | Delay (s) | LOS | v/c |
| Martin/Ottawa | 0.49(0.58) | A(A) | EBT(WBL) | 12.4(12.5) | A(A) | 0.39(0.55) |
| Paterson/Ottawa | 0.62(0.64) | B(B) | NBT(WBT) | 13.1(14.7) | A(A) | 0.50(0.60) |
| Industrial/Ottawa | 0.26(0.63) | A(B) | WBT(WBT) | 10.4(16.8) | A(A) | 0.23(0.56) |

Note: Analysis of signalized intersections assumes a PHF of 0.95 and a saturation flow rate of 1800 veh/h/lane.
As shown in Table 3, the study area intersections currently operate at an excellent LOS ' $A$ ' during the morning and afternoon peak hours. With regard to 'critical movements' at study area intersections, they are operating at an acceptable LOS 'B' or better during peak hours. Therefore, vehicle related congestion is not expected to be a contributing factor to the noted safety concerns.

